

CHAPTER IV

MODEL DESIGN



4.1 Basis of Model Design

Model design has been based on geometric similitude;

1) For geometrical similarity the prototype and the model, it is necessary that all linear dimensions of the model be scaled down from the corresponding dimensions of the prototype by a constant ratio. For this study, the geometrical scale factor is made equal to three.

Model design has been based on stress similitude;

1) Keeping the ratio of the design column load on pile cap, P , to the design shear strength contributed by the concrete, V , constant between the prototype and the model.

2) Keeping the ratio of the calculated flexural strength, M , to the design column, P , constant between the prototype and the model.

4.2 Model Design Analysis

4.2.1 Three-Pile Cap

1) Punching Shear:

prototype;

a) Dimension $b=142$ cm., $c=45$ cm., $d=75$ cm., $L=105$ cm.

b) Material

$$\text{Concrete} \quad f'_c = 250 \text{ ksc.}$$

$$\text{Reinforcement} \quad f_y = 3400 \text{ ksc.}$$

$$f_s = 1700 \text{ ksc.}$$

c) Analysis

$$v = 0.53/\sqrt{f'_c} = 0.53/\sqrt{250} = 8.38 \text{ ksc.}$$

$$V = vb_o d = 8.38 \times 4(45+75)75 = 301,681 \text{ ksc.}$$

$$P = 50 \times 3 \times 1000 = 150,000 \text{ kg.}$$

$$\frac{P}{V} = \frac{150,000}{301,681} = 0.497$$

model;

a) Geometrical scale factor = 3

b) Design analysis

$$\frac{P}{V} \text{ model} = \frac{P}{V} \text{ prototype} = 0.497$$

$$\begin{aligned} P_{\text{model}} &= V_{\text{model}} \times 0.497 = 0.53/\sqrt{f'_c} \left(\frac{b}{3}\right) \left(\frac{d}{3}\right) (0.497) \\ &= \frac{150,000}{9} = 16,667 \text{ kg.} \end{aligned}$$

Force scale factor = 9

2) Flexural Analysis of Model: (Working Stress Design)

a) Beam analogy

$$A_s = \frac{M}{f_s j d} = \frac{16667 \times 12.73}{3 \times 1700 \times 0.876 \times 25} = 1.90 \text{ cm}^2$$

Use 3- ϕ 9 mm. ($A_s = 1.909 \text{ cm}^2$)

b) Truss analogy

$$\begin{aligned} T &= \frac{P(2L^2 - c^2)}{18Ld} \\ &= \frac{16667(2 \times 35^2 - 15^2)}{18 \times 35 \times 25} = 2355 \text{ kg.} \end{aligned}$$

$$A_s = \frac{T}{f_s} = \frac{2355}{1700} = 1.39 \text{ cm}^2$$

Use 3- ϕ 9 mm. ($A_s = 1.909 \text{ cm}^2$)

4.2.2 Four-Pile Cap

1) Punching Shear:

prototype;

- a) Dimension $b=170 \text{ cm.}$, $c=45 \text{ cm.}$, $d=75 \text{ cm.}$, $L=105 \text{ cm.}$
 b) Material

Concrete $f'_c = 250 \text{ ksc.}$

Reinforcement $f_y = 3400 \text{ ksc.}$

$f_s = 1700 \text{ ksc.}$

c) Analysis

$$v = 0.53/\sqrt{f'_c} = 0.53/\sqrt{250} = 0.497$$

$$V = vb_o d = 0.497 \times 4(45+75)75 = 301,681 \text{ kg.}$$

$$P = 50 \times 4 \times 1000 = 200,000 \text{ kg.}$$

$$\frac{P}{V} = \frac{200,000}{301,681} = 0.663$$

model;

a) Geometrical scale factor = 3

b) Design analysis

$$\frac{P}{V} \text{ model} = \frac{P}{V} \text{ prototype} = 0.663$$

$$\begin{aligned} P_{\text{model}} &= V_{\text{model}} \times 0.663 \\ &= 0.53/\sqrt{f'_c} \left(\frac{b_o}{3}\right) \left(\frac{d}{3}\right) (0.663) \\ &= \frac{200,000}{9} = 22,222 \text{ kg.} \end{aligned}$$

Force scale factor = 9

2) Flexural Analysis of Model; (Working Stress Design)

a) Beam Analogy

$$A_s = \frac{M}{f_s j d} = \frac{22222 \times 10}{2 \times 1700 \times 0.876 \times 25} = 2.98 \text{ cm}^2$$

Use 6- ϕ 9mm. ($A_s = 3.817 \text{ cm}^2$)

b) Truss Analogy

$$\begin{aligned} T &= (P/24Ld)(3L^2 - c^2) \\ &= \frac{22222(3 \times 35^2 - 15^2)}{24 \times 35 \times 25} = 3651 \text{ kg.} \end{aligned}$$

$$\begin{aligned} A_s &= \frac{T}{f_s} = \frac{3651}{1700} = 2.15 \text{ cm}^2/\text{edge} \\ &= 2 \times 2.15 = 4.30 \text{ cm}^2/\text{each way} \end{aligned}$$

Use 6- ϕ 9mm. ($A_s = 3.817 \text{ cm}^2$)