

## REFERENCES

- Awojobi, A. O., and Grootenhuis, P. 1965. Vibration of rigid bodies on semi-infinite elastic media. Proceedings of the Royal Society London, England, Vol. 287, Series A : 27-63.
- Biot, M. A. 1941. General theory of three-dimentional consolidation. J. Appl. Physics 12 : 155-164.
- Biot, M. A. 1956. Theory of propagation of elastic waves in a fluid-saturated porous solid. I. Low-frequency range. J. Acoust. Soc. Am 28(2) : 168-178.
- Biot, M. A. 1962. Mechanics of deformation and acoustic propagation in porous media. J. Appl. Physics 33 : 1482-1498.
- Bo, J. and Xu, Z. K. 1997. A dynamic analysis of elastic circular plate on saturated poroelastic half-space. Acta Mechanica Solida Sinica 10(4) : 299-308.
- Bo, J. 1999. The vertical vibration of an elastic circular plate on a fluid-saturated porous half space. Int. J. Engng. Sci 37 : 379-393.
- Bo, J., and Hua, L. 1999. Vertical dynamic response of a disk on a saturated poroelastic half-space. Soil Dyn. and Earthquake Engng 18 : 437-443.
- Bo, J., and Hua, L. 2000. Transient response of an elastic circular plate on a poroelastic half space. Mech. Res. Com 27(2) : 149-156.
- Bougacha, S., Tassoulas, J. L., and Roesset, J. M. 1993. Dynamic stiffness of foundations on fluid-filled poroelastic stratum. J. Engng. Mech. Div., ASCE 119 (8) : 1649-1662.

Dargush, G. F., and Chopra, M. B. 1996. Dynamic analysis of axisymmetric foundations on poroelastic media. J. Engng. Mech. Div., ASCE 122(7) : 623-632.

Dhawan, G. K. 1981. An asymmetric mixed boundary value problem of a transverse isotropic half-space subjected to a moment by an annular rigid punch. Acta Mechanica 38(1-2) : 257-265.

Gucunski, N., and Peek, R. 1993. Vertical vibrations of circular flexibility foundations on layered media. Soil Dyn. and Earthquake Engng 12 : 183-192.

Halpern, M. R., and Christiano P. 1986. Steady-state harmonic response of a rigid plate bearing on a liquid-saturated poroelastic half space. Earthquake Engng. and Struct. Dyn 14 : 439-454.

Hudson, D. E. 1977. Dynamic tests of full-scale structures. J. Engng. Mech. Div., ASCE 103(6) : 1019-1032.

Iguchi, M., and Luco, Y. E. 1982. Vibration of flexible plate on viscoelastic medium. J. Engng. Mech. Div., ASCE 186(6) : 1381-1395.

Japon, B. R., Gallego, R., and Dominguez, J. 1997. Dynamic stiffness of foundations on saturated poroelastic soils. J. Engng. Mech. Div., ASCE 123(11) : 1121-1129.

Kassir, M. K., and Xu, J. 1988. Interaction functions of a rigid strip bonded to saturated elastic half-space. Int. J. Solids Structures 24 : 915-936.

Kassir, M. K., Xu, J., and Bandyopadyay, K. K. 1996. Rotary and horizontal vibrations of a circular surface footing on a saturated elastic half-space. Int. J. Solids Structures 33(2) : 265-281.

- Kausel, E., Roessel, J. M., and Waas, G. 1975. Dynamic analysis of footings on layered media. J. Engng. Mech. Div., ASCE 101 : 85-105.
- Krenk, S., and Schmidt, H. 1981. Vibration of an elastic circular plate on an elastic half space-a direct approach. J. Appl. Mech., ASME 48 : 161-168.
- Lin, Y. E. 1978. Dynamic response of circular plates resting on viscoelastic half space. J. Appl. Mech., ASME 45 : 379-384.
- Luco, J. E., and Westman, R. A. 1971. Dynamic response of circular footing. J. Engng. Mech. Div., ASCE 97(5) : 1381-1395.
- Luco, J. E., and Westman, R. A. 1972. Dynamic response of rigid footing bonded to an elastic half space. J. Appl. Mech., ASME 39 : 527-534.
- Philippacopoulos, A. J. 1989. Axisymmetric vibration of disk resting on saturated layered half space. J. Engng. Mech. Div., ASCE 115(10) : 2301-2324.
- Rajapakse, R. K. N. D. 1989. The interaction between a circular elastic plate and a transversely isotropic elastic half-space. J. Engng. Mech. Div., ASCE 115(9) : 1867-1881.
- Robertson, I. A. 1966. Forced vertical vibration of a rigid circular disk on a semi-infinite elastic solid. Proc., Cambridge Philosophical Society 62A : 547-553.
- Senjuntichai, T., and Rajapakse, R.K.N.D. 1995. Exact stiffness method for quasi-statics of a multi-layered poroelastic Medium. Int. J. Solids Structures 32 : 1535-1553.

Senjuntichai, T., and Rajapakse, R. K. N. D. 1996. Dynamics of a rigid strip bonded to a multi-layered poroelastic half-plane. in A.P.S. Selvadurai (ed.), Mechanics of Poroelastic Media, Kluwer Academic Publisher, Dordrecht, The Netherlands.

Sneddon, I., 1970. The Use of Integral Transforms. New York : McGraw-Hill Book Co.

Tassoulas, J. L., and Kausel, E. 1984. On the dynamic stiffness of circular ring footings on an elastic stratum. Int. for Numer. and Analytical Meth. in Geomech 8(4) :L 411-426.

Timoshenko, S. P., and Woinowsky-Krieger, S. 1959. Theory of plates and shells. New York : McGraw-Hill Book Co.

Veletsos, A. S., and Wei, Y. T. 1971. Lateral and rocking vibrations of footing. J. Soil Mech. and Found. Engng. Div., ASCE 91(9) : 1227-1248.

Veletsos, A. S., and Tang, Y. 1987a. Vertical vibration of ring foundations. Earthquake Engng. and Struct. Dyn 15(1) : 1-21.

Veletsos, A. S., and Tang, Y. 1987b. Rocking vibration of rigid ring foundations. J. Geotech. Engng., ASCE 113(9) : 1019-1032.

Washizu, K. 1982. Variational methods in elasticity and plasticity. 2nd Ed., New York : Pergamon Press

Watson, G. N. 1944. A treatise on the Theory of Bessel Functions. University Press, Cambridge

Whitman, R. V., Protonotarios, J. N., and Nelson, M. F. 1973. Case study of dynamic soil-structure interaction. J. Soil. Mech. and Found. Div., ASCE 99(11) : 997-1009.

Wolfram, S. 1988. Mathematica: A system for doing mathematic by computer : Addison-Wesley Publishing Company, Inc.

Wong, H. L., and Luco, J. E. 1986. Dynamic response of rigid foundations in a layered half space. Soil Dyn. and Earthquake Engng 5 : 149-158.

Zeng, X., and Rajapakse, R. K. N. D. 1999. Vertical vibrations of a rigid disk embedded in a poroelastic medium. Int. for Numer. and Analytical Meth. In Geomech 23(15) : 2075-2095.

Zienkiewicz, O. C. 1997. The finite element method. 3<sup>rd</sup> Ed., New York : McGraw-Hill Book Co.

## **APPENDICES**

## APPENDIX A

This appendix is concerned with the derivation of the general solutions for a homogeneous poroelastic material undergoing axisymmetric vibrations. Consider the governing equations, equations (3.4) and (3.5) in Chapter III. These equations can be solved by introducing the displacement decomposition based on Helmholtz representation for an axisymmetric vector field, equations (3.6) to (3.9), together with the assumption that the motion is time-harmonic yield two sets of partial differential equations for  $\Phi_1, \Phi_2$  and  $\Psi_1, \Psi_2$  as

$$[(\lambda^* + \alpha^2 M^* + 2)\nabla^2 + \delta^2]\Phi_1 = -(\alpha M^* \nabla^2 + \rho^* \delta^2)\Phi_2 \quad (\text{A.1})$$

$$(\alpha M^* \nabla^2 + \rho^* \delta^2)\Phi_1 = (ib^* \delta - m^* \delta^2 - M^* \nabla^2)\Phi_2 \quad (\text{A.2})$$

$$(\nabla^2 + \delta^2)\Psi_1 = -\rho^* \delta^2 \Psi_2 \quad (\text{A.3})$$

$$\rho^* \delta^2 \Psi_1 = (ib^* \delta - m^* \delta^2)\Psi_2 \quad (\text{A.4})$$

where the dimensionless parameters  $\lambda^*$ ,  $M^*$ ,  $\rho^*$ ,  $m^*$  and  $b^*$  are defined as

$$\lambda^* = \frac{\lambda}{\mu}, \quad M^* = \frac{M}{\mu}, \quad \rho^* = \frac{\rho_f}{\rho}, \quad m^* = \frac{m}{\rho} \quad \text{and} \quad b^* = \frac{ab}{\sqrt{\rho\mu}} \quad (\text{A.5})$$

and  $\nabla^2$  is the Laplacian operator defined by

$$\nabla^2 = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{\partial^2}{\partial z^2} \quad (\text{A.6})$$

In addition, a dimensionless frequency,  $\delta$ , is defined as

$$\delta = \sqrt{\frac{\rho}{\mu}} \omega a \quad (\text{A.7})$$

Application of the zeroth-order Hankel transform to equations (A.1) to (A.4) yield the ordinary differential equations for  $\Phi_1, \Phi_2$  and  $\Psi_1, \Psi_2$  as

$$\left[ (\lambda^* + \alpha^2 M^* + 2) \left( \frac{d^2}{dz^2} - \xi^2 \right) + \delta^2 \right] \bar{\Phi}_1 = \left[ \alpha M^* \left( \xi^2 - \frac{d^2}{dz^2} \right) - \rho^* \delta^2 \right] \bar{\Phi}_2 \quad (\text{A.8})$$

$$\left[ \alpha M^* \left( \frac{d^2}{dz^2} - \xi^2 \right) + \rho^* \delta^2 \right] \bar{\Phi}_1 = \left[ ib^* \delta - m^* \delta^2 - M^* \left( \xi^2 - \frac{d^2}{dz^2} \right) \right] \bar{\Phi}_2 \quad (\text{A.9})$$

$$\left( \xi^2 - \frac{d^2}{dz^2} - \delta^2 \right) \bar{\Psi}_1 = \rho^* \delta^2 \bar{\Psi}_2 \quad (\text{A.10})$$

$$\rho^* \delta^2 \bar{\Psi}_1 = (ib^* \delta - m^* \delta^2) \bar{\Psi}_2 \quad (\text{A.11})$$

It can be shown that the general solutions of Hankel transforms of  $\Phi_i$  and  $\Psi_i$  ( $i=1,2$ ) can be expressed as

$$\bar{\Phi}_1 = Ae^{\gamma_1 z} + Be^{-\gamma_1 z} + Ce^{\gamma_2 z} + De^{-\gamma_2 z} \quad (\text{A.12})$$

$$\bar{\Phi}_2 = \chi_1 (Ae^{\gamma_1 z} + Be^{-\gamma_1 z}) + \chi_1 (Ce^{\gamma_2 z} + De^{-\gamma_2 z}) \quad (\text{A.13})$$

$$\bar{\Psi}_1 = Ee^{\gamma_3 z} + Fe^{-\gamma_3 z} \quad (\text{A.14})$$

$$\bar{\Psi}_2 = \chi_3 (Ee^{\gamma_3 z} + Fe^{-\gamma_3 z}) \quad (\text{A.15})$$

where  $A(\xi, \delta), B(\xi, \delta), \dots, F(\xi, \delta)$  are the arbitrary functions to be determined by using appropriate boundary and/or continuity conditions relevant to a given problem and

$$\chi_i = \frac{(\lambda^* + \alpha^2 M^* + 2)L_i^2 - \delta^2}{\rho^* \delta^2 - \alpha M^* L_i^2}, \quad i=1,2 \quad (\text{A.16})$$

$$\chi_3 = \frac{\rho^* \delta}{ib^* - m^* \delta} \quad (\text{A.17})$$

$$\gamma_i = \sqrt{\xi^2 - L_i^2}, \quad i=1,2 \quad (\text{A.18})$$

$$\gamma_3 = \sqrt{\xi^2 - S^2} \quad (\text{A.19})$$

$$L_1^2 = \frac{w_1 + \sqrt{w_1^2 - 4w_2}}{2} \quad (\text{A.20})$$

$$L_2^2 = \frac{w_1 - \sqrt{w_1^2 - 4w_2}}{2} \quad (\text{A.21})$$

$$S^2 = (\rho^* \chi_3 + 1) \delta^2 \quad (\text{A.22})$$

$$w_1 = \frac{(m^* \delta^2 - ib^* \delta)(\lambda^* + \alpha^2 M^* + 2) + M^* \delta^2 - 2\alpha M^* \rho^* \delta^2}{(\lambda^* + 2)M^*} \quad (\text{A.23})$$

$$w_2 = \frac{(m^* \delta^2 - ib^* \delta)\delta^2 - (\rho^*)^2 \delta^4}{(\lambda^* + 2)M^*} \quad (\text{A.24})$$

In view of equations (3.1)-(3.3), (3.6)-(3.9) and (A.12)-(A.15), the general solutions for Hankel transforms of displacements  $u_i$  and  $w_i$  ( $i=r, z$ ), stresses  $\sigma_{ij}$

and excess pore pressure  $p$ , given in equations (3.12) to (3.17), can be obtained. In addition, the variables  $\eta_i$ ,  $\beta_i$  and  $S_1$  are defined by

$$\eta_i = (\alpha + \chi_i) M^* L_i^2, \quad i = 1, 2 \quad (\text{A.25})$$

$$\beta_i = 2\gamma_i^2 - \lambda^* L_i^2 - \alpha \eta_i, \quad i = 1, 2 \quad (\text{A.26})$$

$$S_1 = \xi^2 + \gamma_3^2 \quad (\text{A.27})$$

## APPENDIX B

This appendix is given the non-zero arbitrary functions appearing in the general solutions given by equations (3.12)-(3.17) for different loading cases.

### B.1 Arbitrary Functions for Vertical Loading:

$$A_1 = \frac{\eta_2 e^{-\gamma_1 z'}}{2\mu N_1} \bar{T}_z(\xi) \quad (\text{B.1})$$

$$B_1 = \frac{\eta_2 (v_2 e^{-\gamma_1 z'} + 2\xi^2 v_3 e^{-\gamma_2 z'} - 4\xi^2 S_1 v_1 e^{-\gamma_3 z'})}{2\mu N_1 R} \bar{T}_z(\xi) \quad (\text{B.2})$$

$$C_1 = -\frac{\eta_1 e^{-\gamma_2 z'}}{2\mu N_1} \bar{T}_z(\xi) \quad (\text{B.3})$$

$$D_1 = \frac{\eta_2 (2\xi^2 v_4 e^{-\gamma_1 z'} - v_6 e^{-\gamma_2 z'} + 4\xi^2 S_1 v_1 e^{-\gamma_3 z'})}{2\mu N_1 R} \bar{T}_z(\xi) \quad (\text{B.4})$$

$$E_1 = \frac{\xi v_1 e^{-\gamma_3 z'}}{2\mu \gamma_3 N_1} \bar{T}_z(\xi) \quad (\text{B.5})$$

$$F_1 = \frac{\xi v_2 (v_4 e^{-\gamma_1 z'} - v_3 e^{-\gamma_2 z'}) + \xi v_1 v_7 e^{-\gamma_3 z'}}{2\mu \gamma_3 N_1 R} \bar{T}_z(\xi) \quad (\text{B.6})$$

$$B_2 = B_1 - A_1 e^{2\gamma_1 z'} \quad (\text{B.7})$$

$$D_2 = D_1 - C_1 e^{2\gamma_2 z'} \quad (\text{B.8})$$

$$F_2 = F_1 + E_1 e^{2\gamma_3 z'} \quad (\text{B.9})$$

where

$$v_1 = \eta_1 - \eta_2 \quad (\text{B.10})$$

$$v_2 = \eta_1 \beta_2 - \eta_2 \beta_1 \quad (\text{B.11})$$

$$v_3 = 4\eta_1 \gamma_2 \gamma_3 \quad (\text{B.12})$$

$$v_4 = 4\eta_2 \gamma_3 \gamma_1 \quad (\text{B.13})$$

$$v_5 = S_1 v_2 - \xi^2 (v_3 + v_4) \quad (\text{B.14})$$

$$v_6 = S_1 v_2 + \xi^2 (v_3 + v_4) \quad (\text{B.15})$$

$$v_7 = S_1 v_2 + \xi^2 (v_3 - v_4) \quad (\text{B.16})$$

and

$$N_1 = 2\xi^2 v_1 - v_2 \quad (\text{B.17})$$

$$R = -S_1 v_2 + \xi^2 (v_3 - v_4) \quad (\text{B.18})$$

In the above equations,  $\bar{T}_z(\xi) = p_o a J_1(\xi a) / \xi$  is the zeroth-order Hankel transform of the applied axisymmetric vertical load over a circular area of radius  $a$  and uniform intensity  $p_o$  at  $(z = z')$ .

## B.2 Arbitrary Functions for Applied Fluid Pressure:

$$A_1 = \frac{-(\lambda^* + 2)L_2^2 e^{-\gamma_1 z'}}{2\mu N_1} \bar{P}(\xi) \quad (\text{B.19})$$

$$B_1 = \frac{-(\lambda^* + 2)[v_5 L_2^2 e^{-\gamma_1 z'} + 2\xi^2 \eta_2 (\beta_3 e^{-\gamma_2 z'} - \beta_5 e^{-\gamma_3 z'})]}{2\mu N_1 R} \bar{P}(\xi) \quad (\text{B.20})$$

$$C_1 = \frac{-(\lambda^* + 2)L_1^2 e^{-\gamma_2 z'}}{2\mu N_1} \bar{P}(\xi) \quad (\text{B.21})$$

$$D_1 = \frac{(\lambda^* + 2)[v_6 L_1^2 e^{-\gamma_2 z'} - 2\xi^2 \eta_1 (\beta_4 e^{-\gamma_1 z'} + \beta_5 e^{-\gamma_3 z'})]}{2\mu N_1 R} \bar{P}(\xi) \quad (\text{B.22})$$

$$E_1 = -\frac{\xi(\lambda^* + 2)(L_1^2 - L_2^2)e^{-\gamma_3 z'}}{2\mu \gamma_3 N_1} \bar{P}(\xi) \quad (\text{B.23})$$

$$F_1 = -\frac{\xi(\lambda^* + 2)[v_2(\beta_4 e^{-\gamma_1 z'} - \beta_3 e^{-\gamma_2 z'}) + v_7(L_1^2 - L_2^2)e^{-\gamma_3 z'}]}{2\mu \gamma_3 N_1 R} \bar{P}(\xi) \quad (\text{B.24})$$

$$B_2 = B_1 - A_1 e^{2\gamma_1 z'} \quad (\text{B.25})$$

$$D_2 = D_1 - C_1 e^{2\gamma_2 z'} \quad (\text{B.26})$$

$$F_2 = F_1 + E_1 e^{2\gamma_3 z'} \quad (\text{B.27})$$

where

$$\beta_3 = 4\gamma_2\gamma_3 L_1^2 \quad (\text{B.28})$$

$$\beta_4 = 4\gamma_3\gamma_1 L_2^2 \quad (\text{B.29})$$

$$\beta_5 = 2S_1(L_1^2 - L_2^2) \quad (\text{B.30})$$

In the above equations,  $\bar{P}(\xi) = p_o a J_1(\xi a) / \xi$  is the zeroth-order Hankel transform of the applied fluid pressure over a circular area of radius  $a$  and uniform intensity  $p_o$  at  $(z = z')$ .

## BIOGRAPHY

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