

CHAPTER III

### NUMERICAL PROCEDURE

# Boundary Discretization

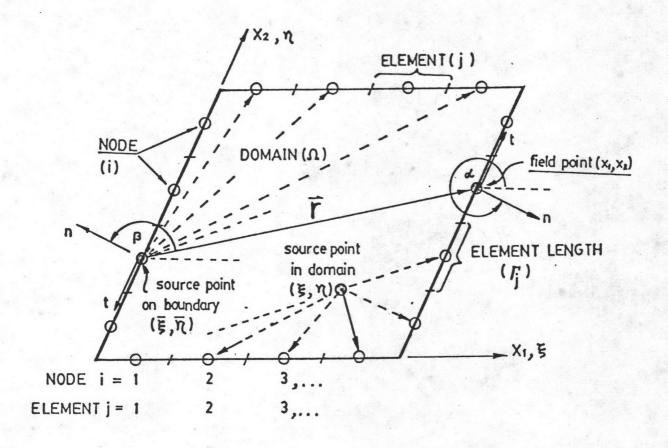


FIG.6 Boundary Nodes , Elements and Sign Convention

The integral equations (28) and (30) can be discretized into a series of elements. The boundary of plate is devided into a series of "segments" or "boundary element" as shown in FIG.6. The points whose unknown values must be determined are called "nodes (i)"

and are taken to be in the middle of each element (j). The unknown function ( W[X\_1,X\_2], N[X\_1,X\_2], M[X\_1,X\_2], V[X\_1,X\_2]) are discretely assumed to be constant values over the interval of element ( $\Gamma_j$ ). Thus the integrals equation (27), (28) and (30) can be approximate by summations. Hence, the values at nodes ( $\bar{\mathbf{F}}_j$ ,  $\bar{\mathbf{N}}_j$ ) in equation (28) and (30) can be calculated. In the case of skew plate, there are four corners, therefore  $j=1,2,\ldots,n+4$ , where n is the total number of boundary elements.

The source points on the boundary node (i) can also be replaced numerically by a set of 2n+4 equations (n+4 equations for equation (28) and the others for equation (30) ) with 4n+8 unknowns as follow:

deflection	W.	; at nodes	i = 1, 2,, n+4
normal slope	N,	; at nodes	i = 1,2,,n
bending moment	M,	; at nodes	i = 1,2,,n
kirchoff's shear	V,	; at nodes	i = 1,2,,n
corner force	R	; at nodes	i = 1,2,,4

But 2n+4 of these unknowns can found from boundary conditions.

Thus , the number of 2n+4 algebraic equations are sufficient .

Rewriting of equation (27) , (28) and (30) yields

$$W(\xi,\eta) = -\sum_{i=1}^{N} W_{i}(X_{i},X_{2}) \int_{\Gamma_{i}} \tilde{V}(X_{i},X_{2};\xi,\eta) d\Gamma_{j}(X_{i},X_{2})$$

$$j = 1$$

$$\begin{array}{ll}
n \\
+ \sum_{i=1}^{N_{i}} (X_{i}, X_{i}) \int_{\Gamma_{i}} M(X_{i}, X_{i}; \epsilon, \eta) d\Gamma_{i}(X_{i}, X_{i}) \\
j = 1
\end{array}$$

$$\sum_{j=1}^{N} (X_{i}, X_{2}) \int_{C_{j}} (X_{i}, X_{2}; \xi, \eta) d\Gamma_{j}(X_{i}, X_{2})$$

$$\begin{array}{ll}
n \\
+ \sum_{i} V_{i} (X_{i}, X_{i}) \int_{\Gamma_{i}} \tilde{W}(X_{i}, X_{i}; \xi, \eta) d\Gamma_{i} (X_{i}, X_{i}) \\
j = 1 \\
n+4 \\
+ \sum_{i} R_{i} (X_{i}, X_{i}) \tilde{W}_{i} (X_{i}, X_{i}; \xi, \eta) \\
j = n+1
\end{array}$$

$$-\sum_{\mathbf{y}_{1}\in\mathbf{X}_{1},\mathbf{X}_{2}}\mathbf{y}_{1}^{*}\mathbf{x}_{1}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{5}^{*}\mathbf{x}_{1}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{5}^{*}\mathbf{x}_{1}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{2}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{4}^{*}\mathbf{x}_{3}^{*}\mathbf{x}_{4}^$$

+ 
$$\int_{\alpha} q \hat{W}(X_1, X_2; \epsilon, \eta) d\Omega(X_1, X_2)$$
 (31)

$$\frac{\mathbf{x}}{2\pi} \quad \mathbf{w}(\bar{\mathbf{x}}_{1}, \bar{\mathbf{\eta}}_{1}) = -\sum_{i=1}^{n} \mathbf{w}_{1} \mathbf{x}_{1}, \mathbf{x}_{2} \mathbf{x}_{1}, \mathbf{x}_{2} \mathbf{x}_{1}, \mathbf{x}_{2}; \bar{\mathbf{x}}_{1}, \bar{\mathbf{\eta}}_{1} \mathbf{x}_{1}, \mathbf{x}_{2} \mathbf{x}_{2}$$

$$\mathbf{j} = 1$$

$$-\sum_{M} (X_1 X_2) \int_{C_1} \tilde{N}(X_1, X_2; \bar{\epsilon}_1, \bar{\eta}_1) d\Gamma_1(X_1, X_2)$$

$$j = 1$$

$$n+4 + \sum_{i=1}^{n+4} R_{i}(X_{i}, X_{2})^{\frac{1}{n}}(X_{i}, X_{2}; \overline{\epsilon}_{i}, \overline{\eta}_{i}) - \sum_{i=1}^{n+4} K_{i}(X_{i}, X_{2})^{\frac{1}{n}}(X_{i}, X_{2}; \overline{\epsilon}_{i}, \overline{\eta}_{i})$$

$$j = n+1 \qquad j = n+1$$

+ 
$$\int_{\Omega} q^{\frac{\pi}{N}} (X_1, X_2; \bar{\epsilon}_1, \bar{\eta}_1) d\Omega(X_1, X_2)$$
 ;  $i = 1, 2, ..., n+4$  (32)

$$\frac{y}{2\pi} = -\sum_{i=1}^{\infty} w_{i}(x_{i}, x_{i}) = -\sum_{i=1}^{\infty} w_{i}(x_{i}, x_{i}) = -\sum_{i=1}^{\infty} \frac{a_{i}(x_{i}, x_{i})}{a_{i}(x_{i}, x_{i})}$$

$$F = \sum_{i=1}^{N} a_{i}(X_{i}, X_{i}) \int_{\Gamma_{i}} a_{i}(X_{i}, X_{i}; \bar{\epsilon}_{i}, \bar{\eta}_{i}) d\Gamma_{i}(X_{i}, X_{i})$$

$$J = 1 \qquad an(\bar{\epsilon}, \bar{\eta})$$

$$j = 1 \qquad \frac{\partial_{x}(x', x')}{\partial y(x', x')} = 1 \qquad \frac{\partial_{x}(x', x')}{\partial y(x', x')}$$

$$n+4$$

$$-\sum_{i=n+1}^{\infty} [X_{i}, X_{2}] a \overline{k}_{j} [X_{i}, X_{2}; \overline{k}_{i}, \overline{\eta}_{i}]$$

$$j = n+1 \qquad \overline{an(\overline{k}, \overline{\eta})}$$

$$+ \int_{\alpha}^{\alpha} q \frac{\partial_{i}^{k}(X_{1}, X_{2}; \bar{\xi}_{1}, \bar{\eta}_{1}) d\Omega(X_{1}, X_{2})}{\partial n(\bar{\xi}_{1}, \bar{\eta}_{2})} ; i = 1, 2, ..., n$$
 (33)

All of the virtual plate terms  $(\tilde{\mathbf{V}}, \tilde{\mathbf{M}}, \dots, \text{etc.})$  are determined by substitute equation (26) into equations (17), (18), (19) and (20) and the normal derivative of equations (17), (18), (19) and (20) (as shown in appendix(C)) as a functions of  $\mathbf{r}^2 \ln(\mathbf{r})$ ,  $\mathbf{r} \ln(\mathbf{r})$ ,  $\ln(\mathbf{r})$ ,  $\ln(\mathbf{$ 

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However , these terms ;  $\ln(r)$  , 1/r and  $1/r^2$  also occur in the expressions for M , V ,  $\partial N/\partial n$  ,  $\partial M/\partial n$  and  $\partial V/\partial n$ . In order to cope with these integral problems , the treatment of singularities by equilibrium equation and analysis method are introduced in CHAPTER IV.

## Matrix Formulation

Since the equation (32) will be expressed in n+4 equations (total boundary elements "n " and 4 corners ) and also another n equations from expression (33) ,therefore it will be more convenient to write these equations in matrix form respectively:

$$[v]_{[V]} = [v]_{[V]} = [v]_$$

$$(\sqrt[4]{3})^{-1} = (\sqrt[4]{3})^{-1} = (\sqrt[4$$

where

[W],[N],[M],[V] = (n+4)xn known matrices of deflection, normal slope,

bending moment and Kirchoff's shear force at

boundary of virtual plate respectively.

 $[\mathring{W}_{c}], [\mathring{R}]$  = (n+4)x4 known matrices of deflection at corner and corner force of real plate.

[W],[N],[M],[V] = nxn known matrices of derivative terms of

deflection, normal slope, bending moment and Kirchoff's shear force at boundary of virtual plate respectively.

- [Wc],[R] = 4x4 known matrices of derivative terms of deflection at corner and corner force of real plate.
  - [0] = (n+4)x1 known matrices of the domain integral term of equation (32)
  - [0] = nx1 known matrices of the domain integral term of equation (33)

Notes : the element of matrices

$$\tilde{V}_{i,i} = \tilde{V}_{i,i}$$

$$\tilde{M}_{i,i} = \tilde{M}_{i,i}$$
and
$$\tilde{V}_{i,i} = \tilde{V}_{i,i} + \frac{1}{2\pi}$$

$$\tilde{M}_{i,i} = \tilde{M}_{i,i} - \frac{1}{2\pi}$$

$$\tilde{M}_{i,i} = \tilde{M}_{i,i} - \frac{1}{2\pi}$$
when  $i = j$ 

The expressions (34) and (35) respresent the sets of boundary integral equations relating the boundary factors on boundary to the values of deflection ,normal slope , bending moment and Kirchoff's shear at any point to one another , according to the boundary

### condition of the plate as follows:

A similar technique can be used for mixed boundary conditions as shown in the example III and IV. Therefore, for any boundary condition only 2n+4 algebraic equations are required to cope with the 2n+4 unknowns. Hence, one can re-replace these set of equations in the form of

$$[A][B] = [C]$$
 (42)

where

[A] = coefficient matrix [(2n+4)x(2n+4)] of linear equations

[B] = unknown matrix [ (2n+4)x1 ]

[C] = known matrix [ (2n+4)x1 ] of domain integral

For example , in the case of all edges are clamped , one can write equation (42) as :

$$\begin{bmatrix} c_{N3} & -c_{M3} & -c_{M}^{c_{3}} \\ c_{N3} & -c_{M3} & -c_{M}^{c_{3}} \end{bmatrix} \begin{bmatrix} c_{N3} \\ c_{N3} \end{bmatrix} = \begin{bmatrix} c_{03} \\ c_{03} \end{bmatrix}$$

The unknowns in equation (42) can be calculated using "Gauss Elimination Method".

#### Domain Solution

By the "Gauss Elimination Method", all of the boundary function are known. Therefore, one can easily determine the values of deflections at any point inside the domain by equation (31).

Finally, the desired stress resultants inside the domain can be obtained by appropriate differentiating of the deflection W[\$, N] in equation (31).